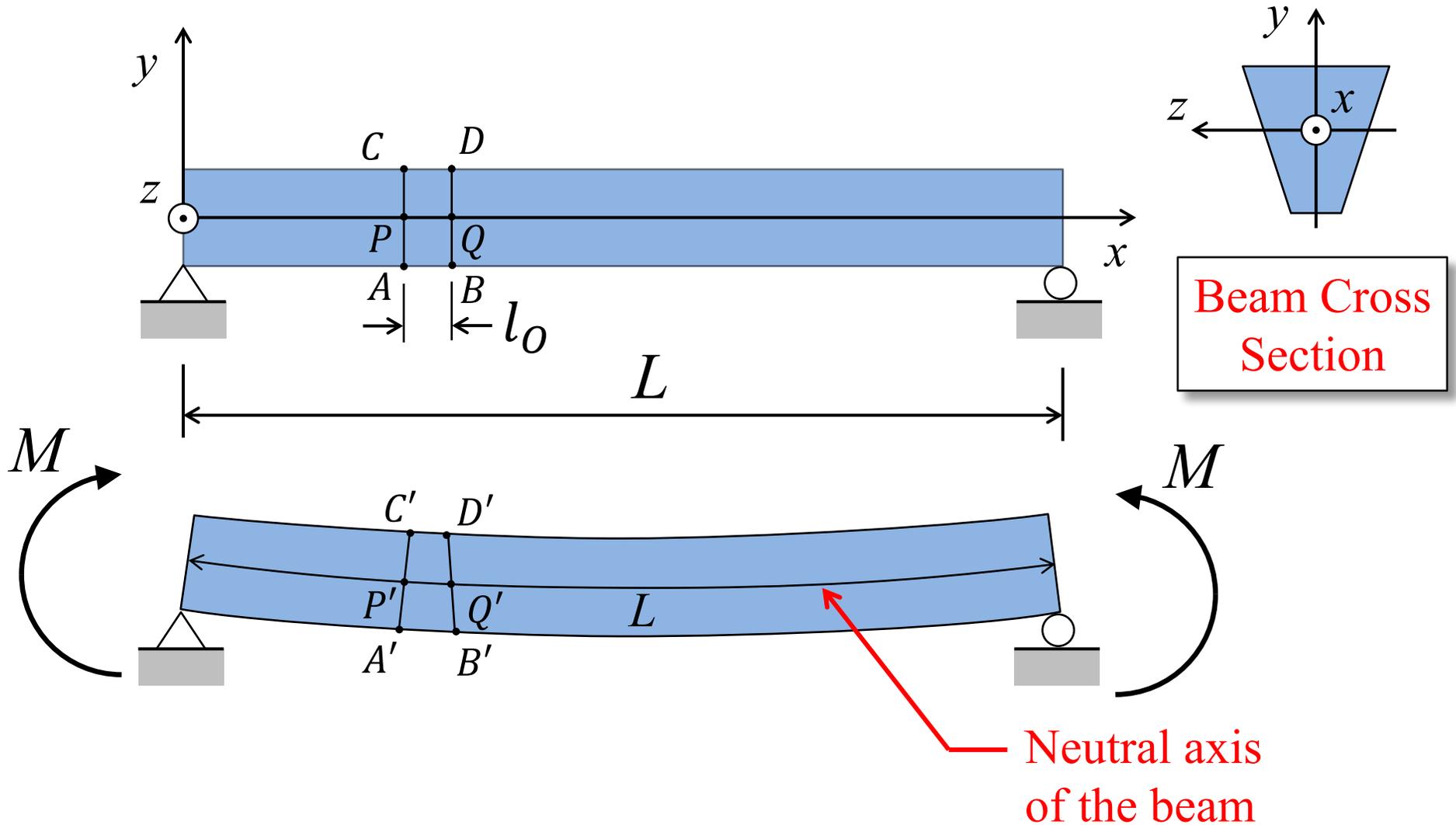


Pure Bending

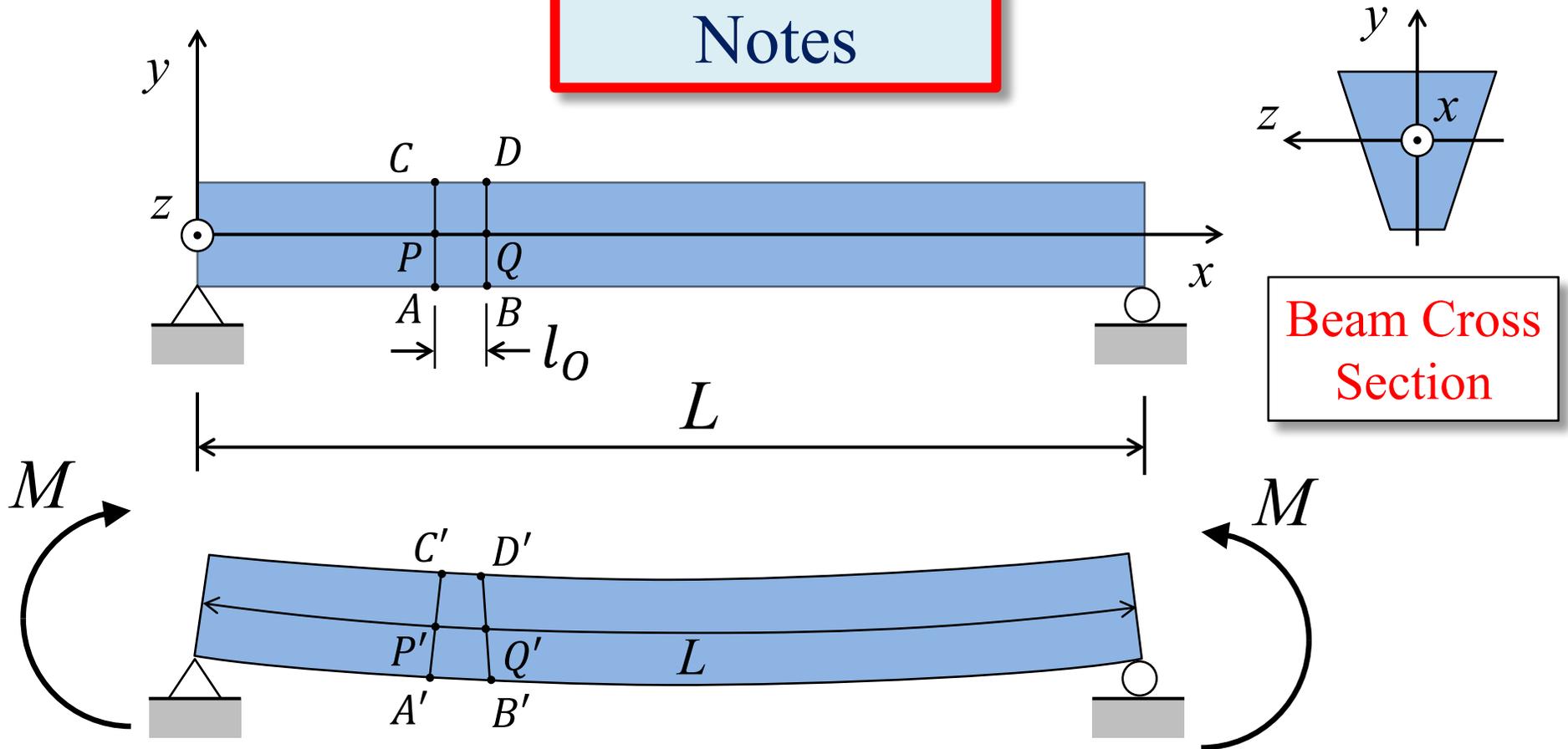
Steven Vukazich

San Jose State University

Consider an Elastic Beam Subjected to Pure Bending



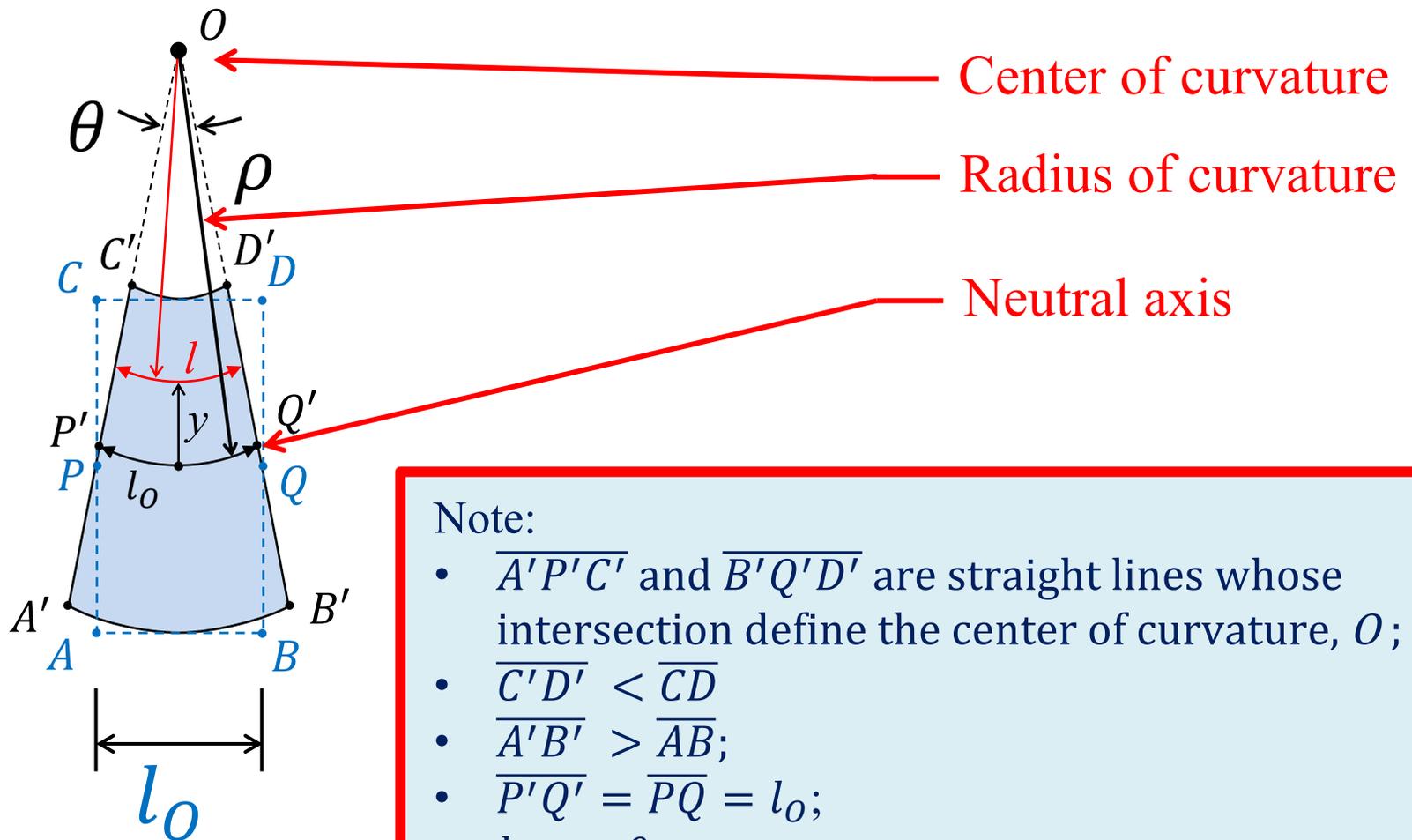
Notes



Beam Cross
Section

1. Beam is prismatic and symmetric about the y axis;
2. Material is linear elastic;
3. The x axis is attached to the neutral axis of the beam;
4. Pure bending (no internal shear) –the beam deforms in a circular arc.

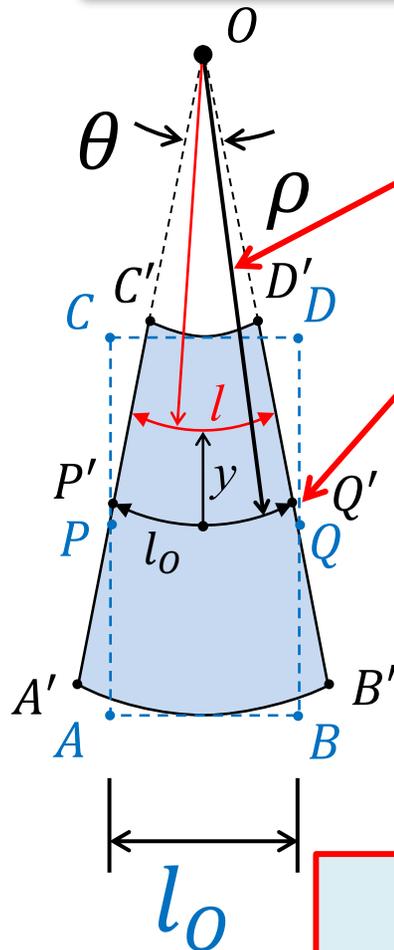
Study the Geometry of the Deformed Shape of a Small Slice of the Beam



Note:

- $\overline{A'P'C'}$ and $\overline{B'Q'D'}$ are straight lines whose intersection define the center of curvature, O ;
- $\overline{C'D'} < \overline{CD}$
- $\overline{A'B'} > \overline{AB}$;
- $\overline{P'Q'} = \overline{PQ} = l_0$;
- $l_0 = \rho\theta$;
- $l = (\rho - y)\theta$

Bending Strain of the Horizontal Fibers of the Beam



Radius of curvature

Neutral axis

Original length of all fibers, l_0

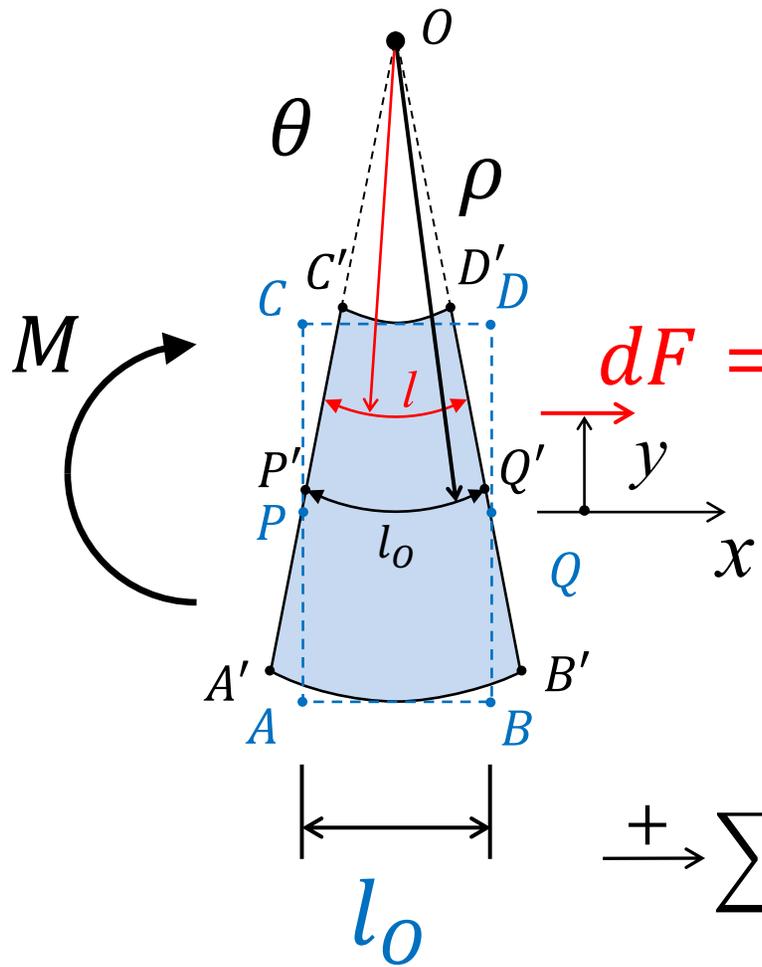
$$l_0 = \rho\theta$$

Length of fiber, l , after deformation

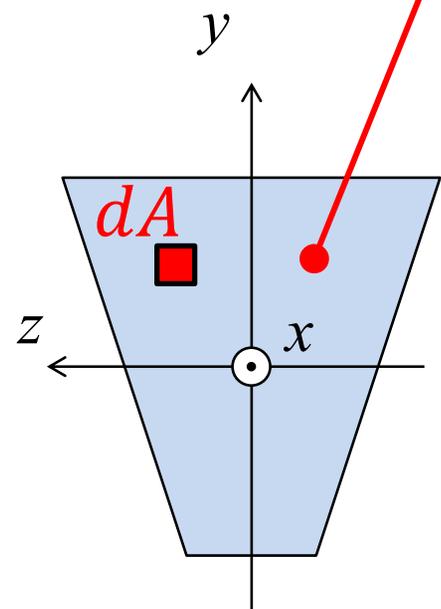
$$l = (\rho - y)\theta$$

$$\epsilon = \frac{\Delta l}{l_0} = \frac{l - l_0}{l_0} = \frac{(\rho - y)\theta - \rho\theta}{\rho\theta} = -\frac{y}{\rho}$$

Equilibrium of the Small Segment of Beam



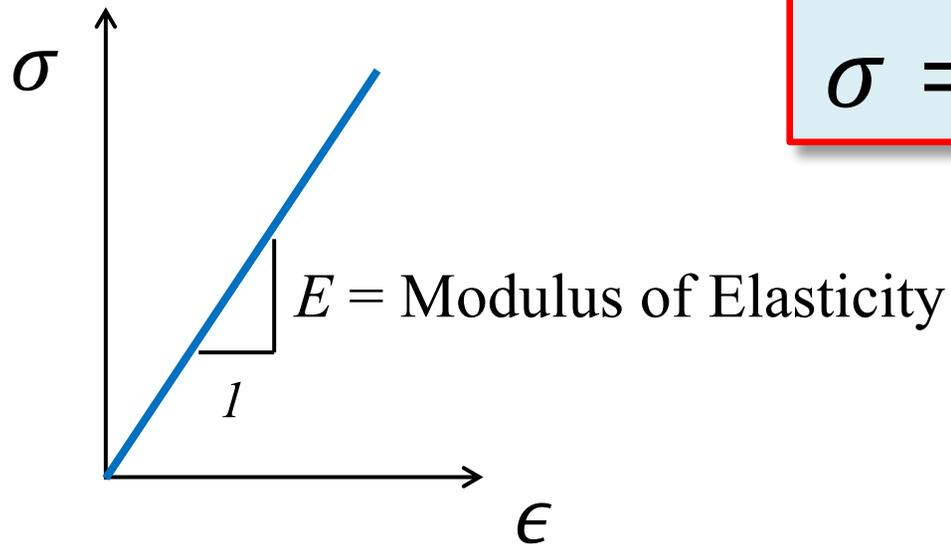
Beam cross sectional area = A



$\overset{+}{\rightarrow} \sum F_x = 0$	$\iint_A dF = 0$	$\iint_A \sigma dA = 0$
$\overset{+}{\curvearrowright} \sum M_z = 0$	$-\iint_A y dF - M = 0$	$-\iint_A y \sigma dA - M = 0$

Constitutive Law for Beam Material

Beam is made of linear elastic material



$$\sigma = E\epsilon$$

Review Relationships from Geometry of Deformation, Equilibrium, and Constitutive Law

1.

$$\epsilon = -\frac{y}{\rho}$$

2.

$$\iint_A \sigma dA = 0$$

$$-\iint_A y\sigma dA - M = 0$$

3.

$$\sigma = E\epsilon$$

Substituting equation 1 into equation 3

$$\sigma = -E\left(\frac{y}{\rho}\right)$$

Force Equilibrium in x Direction

$$\iint_A \sigma dA = 0$$

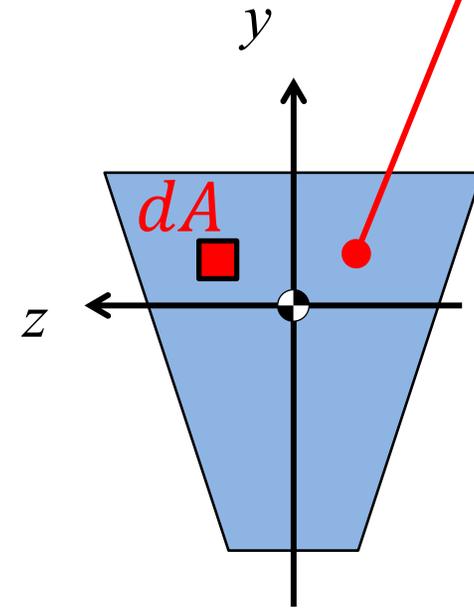
Beam cross sectional area = A

$$\sigma = -E \left(\frac{y}{\rho} \right)$$

$$\iint_A -E \left(\frac{y}{\rho} \right) dA = 0$$

$$-\frac{E}{\rho} \iint_A y dA = 0$$

$$\iint_A y dA = 0$$



Can only be satisfied if the neutral axis is at the **centroid** of the beam cross section

Moment Equilibrium

$$-\iint_A y\sigma dA - M = 0$$

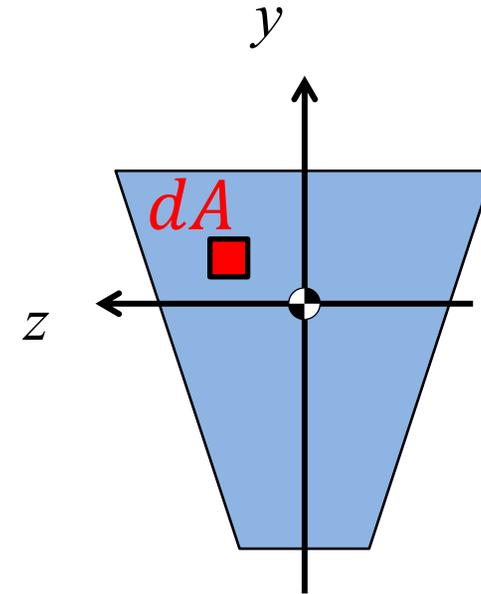
$$\sigma = -E \left(\frac{y}{\rho} \right)$$

$$-\iint_A -yE \left(\frac{y}{\rho} \right) dA - M = 0$$

$$\frac{E}{\rho} \iint_A y^2 dA - M = 0$$

$$M = \frac{E}{\rho} \iint_A y^2 dA$$

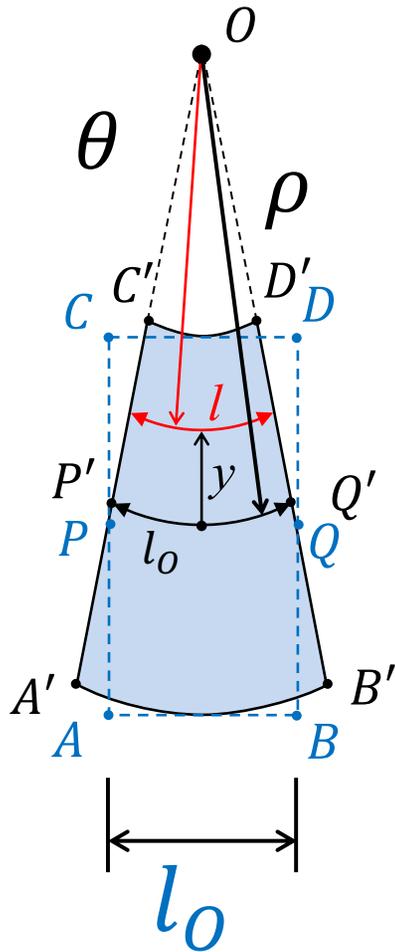
$$M = \frac{EI}{\rho}$$



Moment of Inertia about the centroid of the beam cross section

$$I = \iint_A y^2 dA$$

Moment-Curvature Relationship



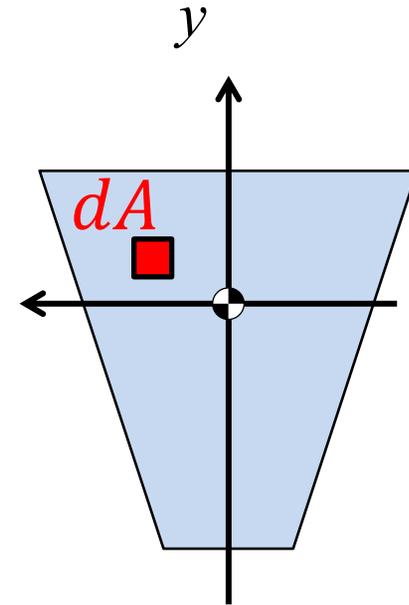
$$M = \frac{EI}{\rho}$$

Define:

$\kappa =$ Curvature of the beam

$$\kappa = \frac{1}{\rho}$$

$$\kappa = \frac{M}{EI}$$

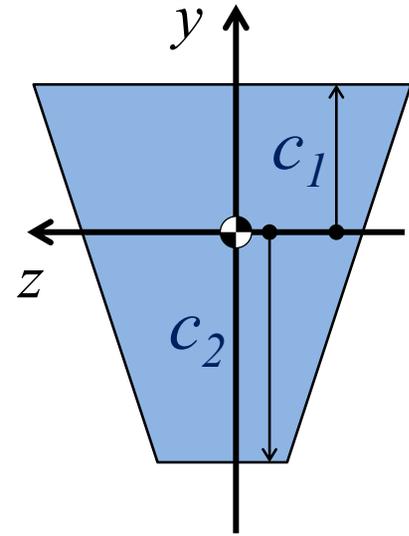
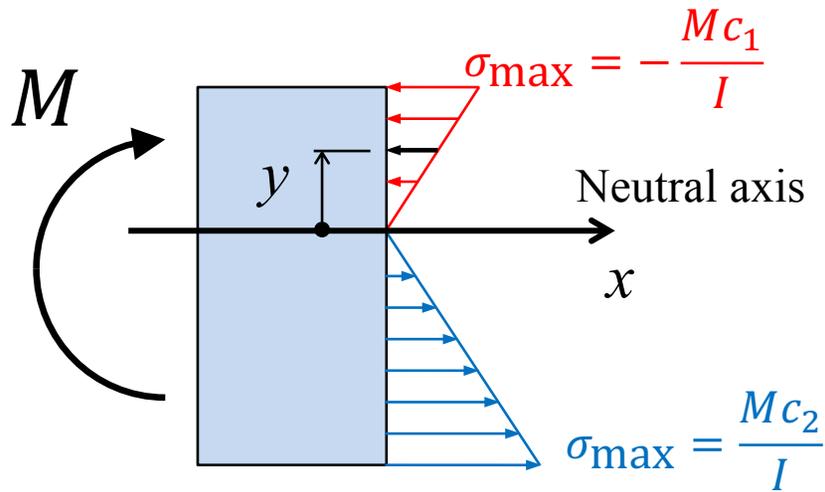


Moment of Inertia about the centroid of the beam cross section

$$I = \iint_A y^2 dA$$

The moment-curvature relationship is the basis of bending deformation theory

Bending Stress Distribution



$y_{\max} = c$

$$M = \frac{EI}{\rho}$$

$$\sigma = -E \left(\frac{y}{\rho} \right)$$

$$\frac{1}{\rho} = \frac{M}{EI}$$

$$\sigma = -Ey \left(\frac{1}{\rho} \right) = -Ey \left(\frac{M}{EI} \right)$$

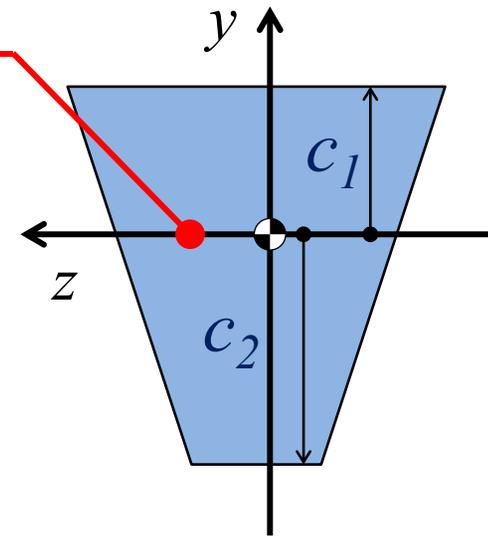
$$\sigma = -\frac{My}{I}$$

Summary for Pure Bending of an Elastic Beam

Moment-Curvature relationship

$$\kappa = \frac{M}{EI}$$

Neutral axis



Bending stress distribution

$$\sigma = -\frac{My}{I}$$

$$\sigma_{\max} = \frac{Mc}{I}$$

1. Neutral axis ($\sigma = 0$) is located at the centroid of the beam cross section;
2. Moment-Curvature relationship is basis of bending deformation theory;
3. Bending stress varies linearly over beam cross section and is maximum at the extreme fibers of the beam;